

Refining Binder Selection to Increase Performance

Mark D Blow, P.E. - Asphalt Institute



**NORTH DAKOTA
ASPHALT
CONFERENCE**

March 2-3, 2022 • Baymont Inn and Suites Mandan

Grading System Based on Climate

PG 58-22

Performance
Grade

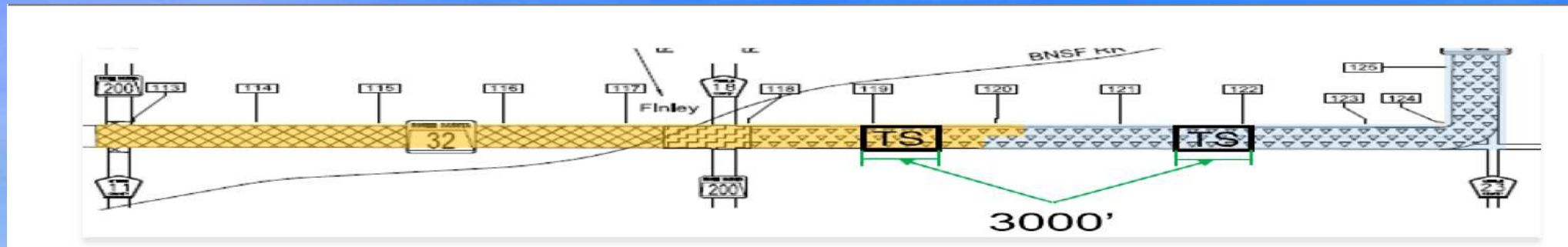
Max pavement
design temp

Min pavement
design temp



MS-2
Pg. 5 – 2.1.1

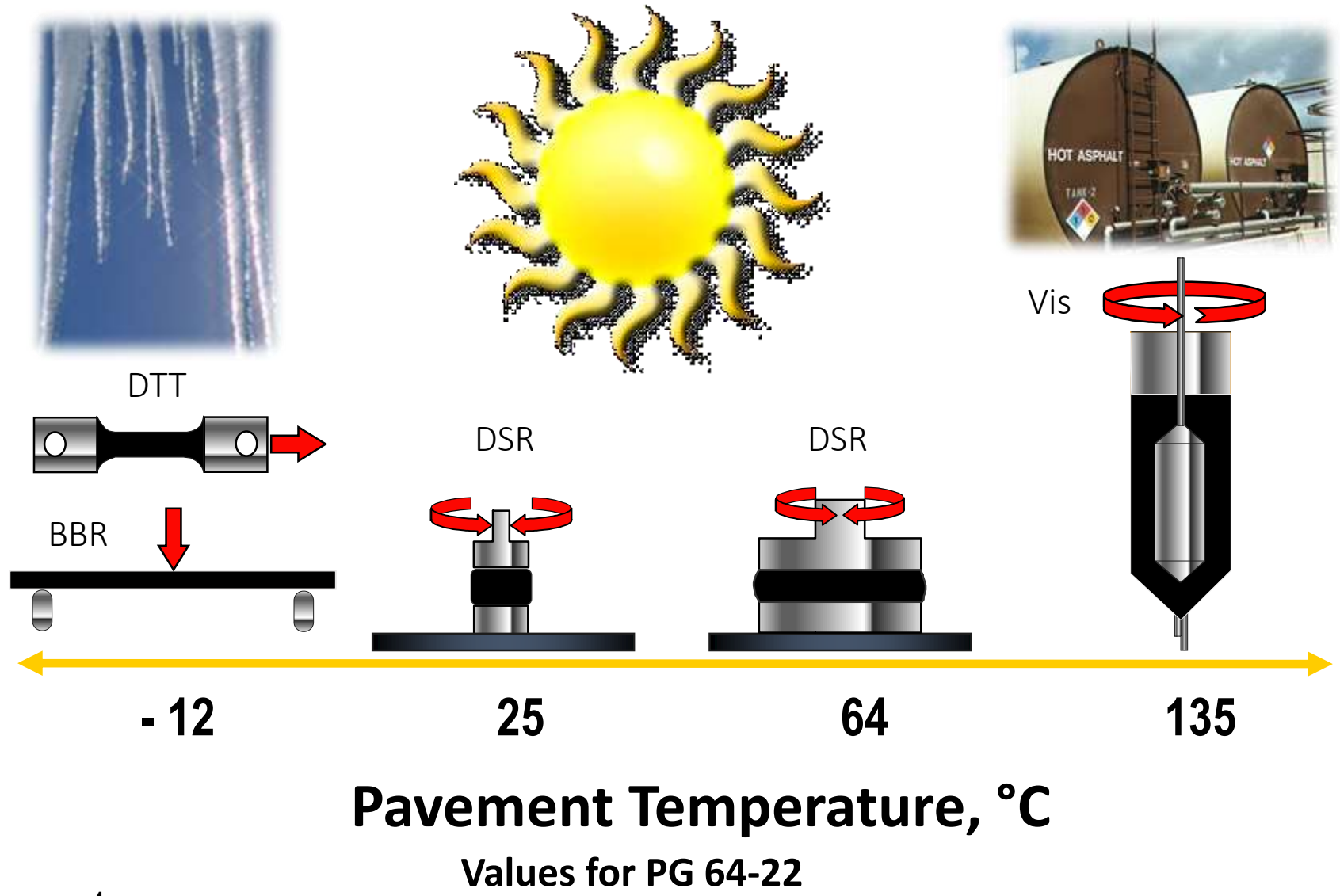
Great work at NDDOT



HWY 32 Pavement Test Sections



Testing Temperature



Performance Grades (AASHTO M320)



High PG	PG 52						PG 58				PG 64				PG 70				PG 76										
Low PG	-10	-16	-22	-28	-34	-40	-46	-16	-22	-28	-34	-40	-10	-16	-22	-28	-34	-40	-10	-16	-22	-28	-34	-40	-10	-16	-22	-28	-34
Original																													
≥ 230 °C	Flash Point , AASHTO T 48																												
≤ 3 Pa·s	Rotational Viscosity @ 135° C, AASHTO T 316																												
≥ 1.00 kPa	DSR $G^*/\sin \delta$ (Dynamic Shear Rheometer), AASHTO T 315																												
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PAV (Pressure Aging Vessel) , AASHTO R28																													
	90						100				100				100(110)				100(110)										
≤ 5000 kPa	DSR $G^*\sin \delta$ (Dynamic Shear Rheometer), AASHTO T 315 Intermediate Temp. = ((High PG + Low PG)/2) + 4																												
	25	22	19	16	13	10	7	25	22	19	16	13	31	28	25	22	19	16	34	31	28	25	22	19	37	34	31	28	25
$S \leq 300$ MPa $m \geq 0.300$	BBR S (creep stiffness) & m-value (Bending Beam Rheometer), AASHTO T 313																												
	0	-6	-12	-18	-24	-30	-36	-6	-12	-18	-24	-30	0	-6	-12	-18	-24	-30	0	-6	-12	-18	-24	-30	0	-6	-12	-18	-24
<ul style="list-style-type: none"> • If BBR m-value ≥ 0.300 and creep stiffness is between 300 and 600, the Direct Tension failure strain requirement of $\geq 1.00\%$ can be used in lieu of the creep stiffness requirement. • Binder shall be homogeneous, free from water, contain no deleterious materials, be at least 99.0% soluble and contain no particles larger than 250 μm. 																													



Example:

- Toll road
PG 64-22
- Toll booth
PG 70-22
- Weigh stations
PG 76-22

← 90 kph (55 MPH)

← Slow

← Stopping

Does PG Grading Predict Performance?

Study of the two mixes with the same aggregate structure, but different binders.

PG 64-22 modified, no rutting



PG 67-22 unmodified, 15mm rut



What happened as a result of M 320's inability to fully characterize polymer modified binders?

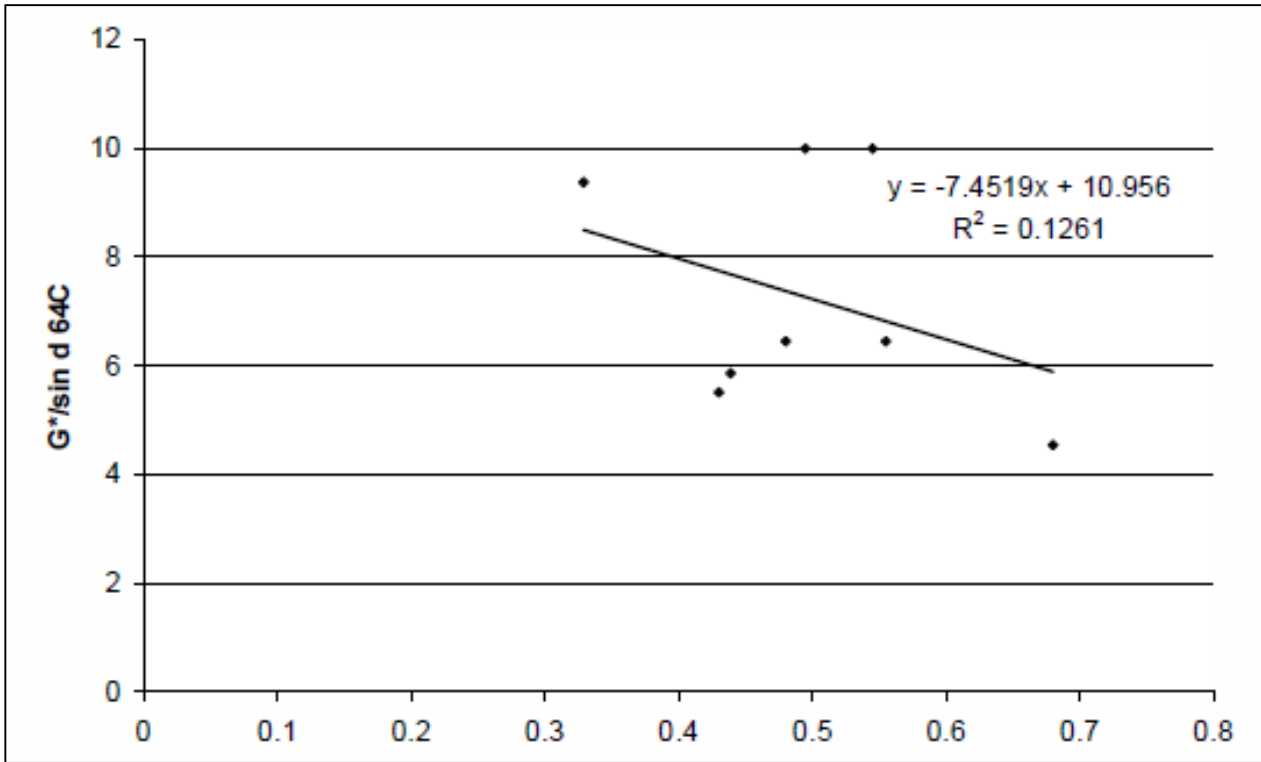
- Most states began requiring additional tests to the ones required in AASHTO M 320
- These mostly empirical tests are commonly referred to as “PG Plus” tests
- These tests are not standard across the states – difficult for suppliers
- Even some of the tests that are the most common, e.g. Elastic Recovery, are not run the same way from state to state

Accelerated Loading Facility (ALF)

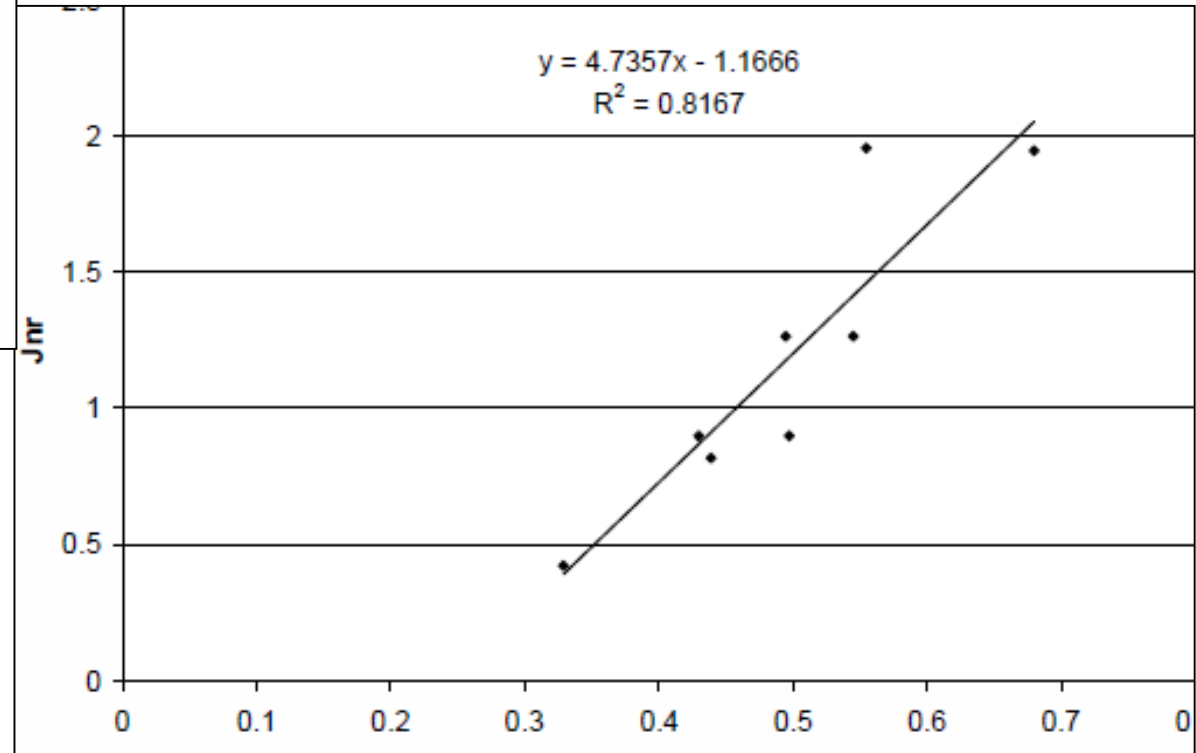
- The pavement was heated to a constant 64°C
- The FHWA ALF uses an 18,000 lbs. single wheel load with no wheel wander
- The speed is 12 MPH
- This is an extreme loading condition far more severe than any actual highway



ALF Loading – M 320 vs. M 332



Traditional M 320
PG Spec
 $R^2 = 0.13$



New M 332
PG Spec
 $R^2 = 0.82$



Multiple Stress Creep Recovery Test

- Performed on RTFO-aged Binder
- Test Temperature
 - Environmental Temperature
 - Not Grade-Bumped
- 10 cycles per stress level
 - 1-second loading at specified shear stress
 - 0.1 kPa
 - 3.2 kPa
 - 9-second rest period



Performance Grades (AASHTO M332)

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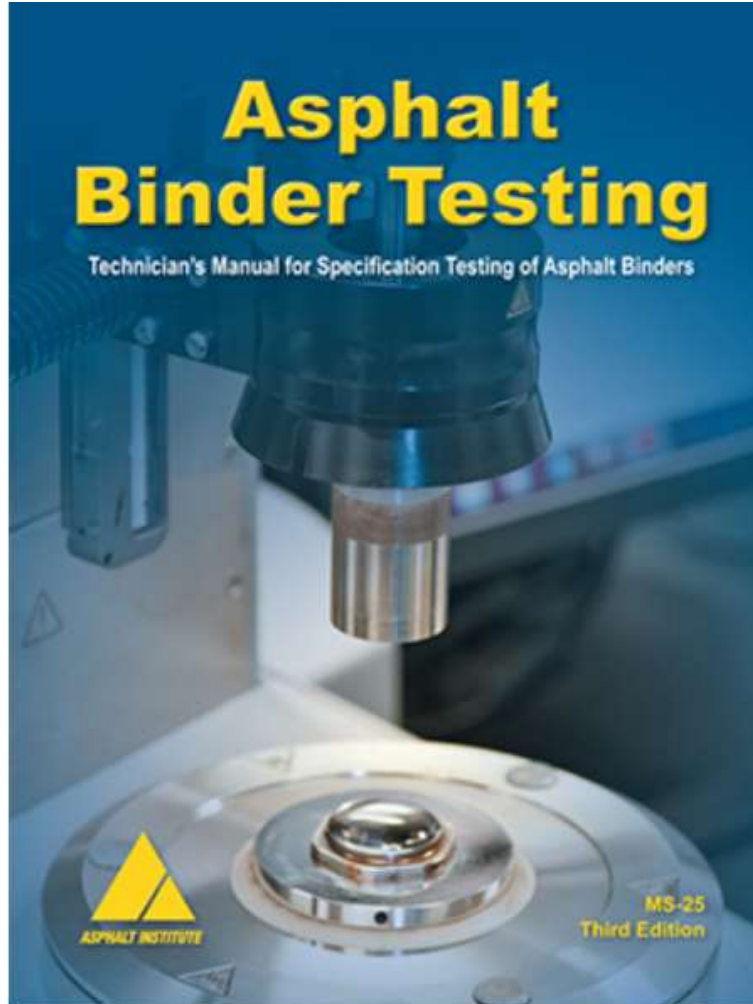
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MSCR PG Grading System (AASHTO M 332)

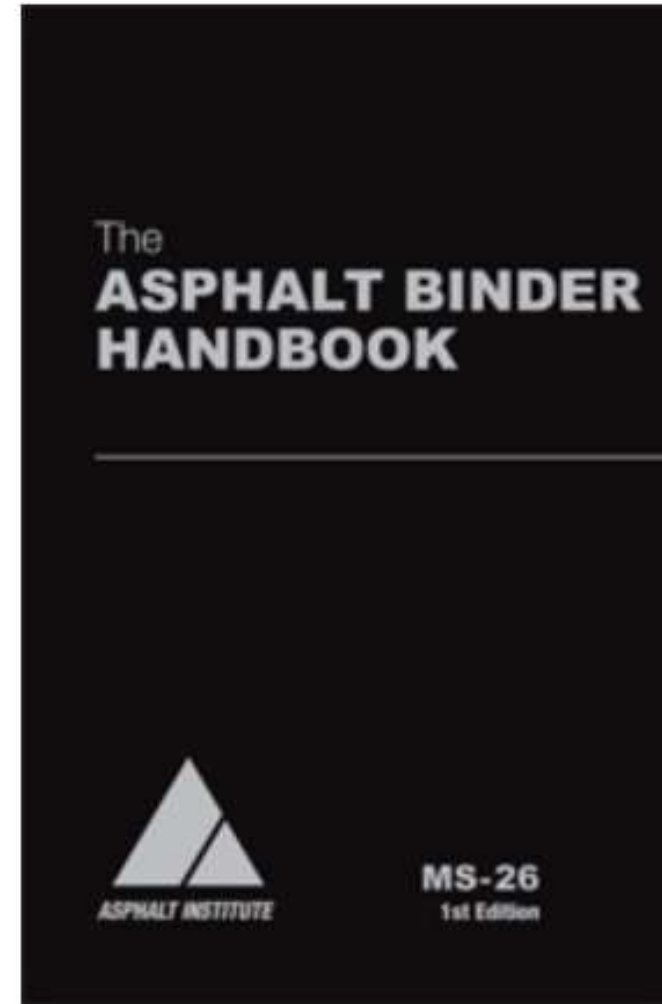
- Environmental grade plus traffic level designation; i.e. **PG 64E-22**
 - Four traffic levels
 - S = Standard: < 10 million ESALs and standard traffic loading
 - H = Heavy: 10 – 30 million ESALs or slow moving traffic loading
 - V = Very Heavy: > 30 million ESALs or standing traffic loading
 - E = Extra Heavy: > 30 million ESALs and standing traffic loading

Note: MSCR system replaces grade bumping.

For More Binder Information



MS-25



MS-26

Is a PG a Modified Binder?

Effect of Loading Rate

Reliability



“Rule of 92”

PG 64 - 34 => 64 - - 34 = 98

Probably modified

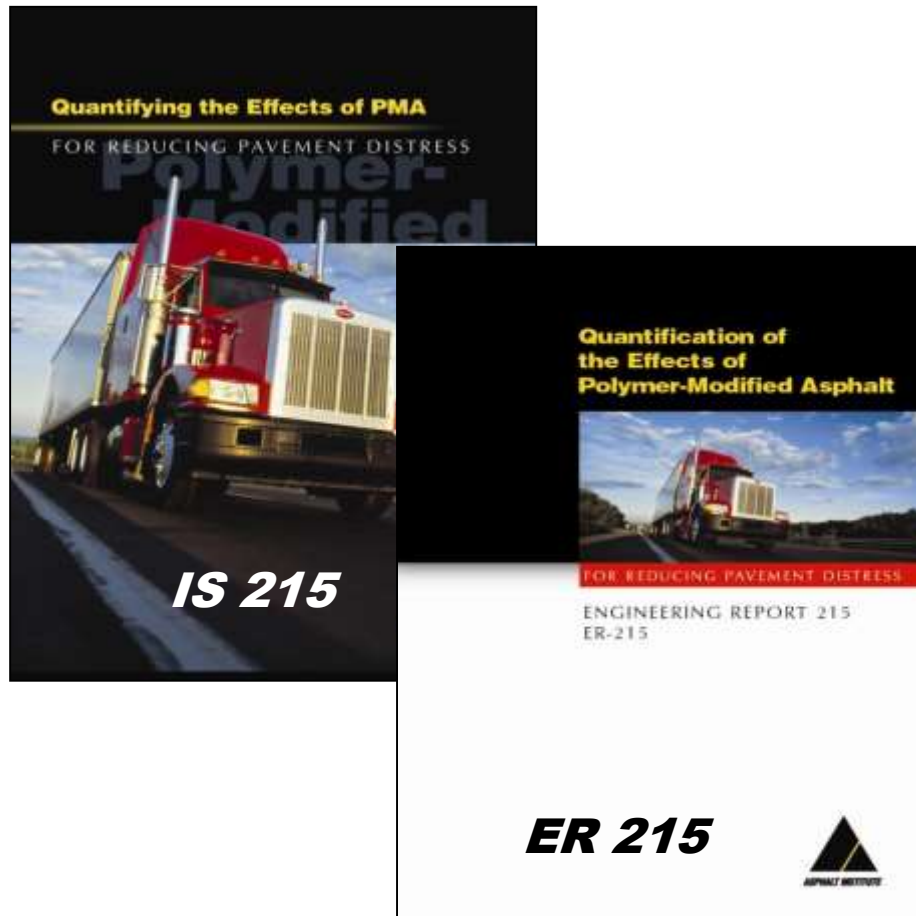
Depends on asphalt source

Rounding

Effect of Traffic

***What are we
looking for-
Price or
Performance?***

Quantifying the Effects of PMA for Reducing Pavement Distress



This study (published in Feb 2005) used national field data to determine enhanced service life of pavements containing polymer modified binders versus conventional binders. The data is from a variety of climates and traffic volumes within North America.

IS-215 Survey Data

Table 1. Summary of Reasons Why Agencies Use PMA Mixtures

Reason for Using PMA Mix	Rutting	Thermal Cracking	Fatigue Cracking	Stripping, Moisture Damage	Durability	Raveling	Tenderness
Primary	58%	21%	0%	0%	0%	0%	0%
Secondary	37%	47%	0%	21%	47%	11%	11%
Total	95%	68%	0%	21%	47%	11%	11%

IS-215 Rut Depth Data

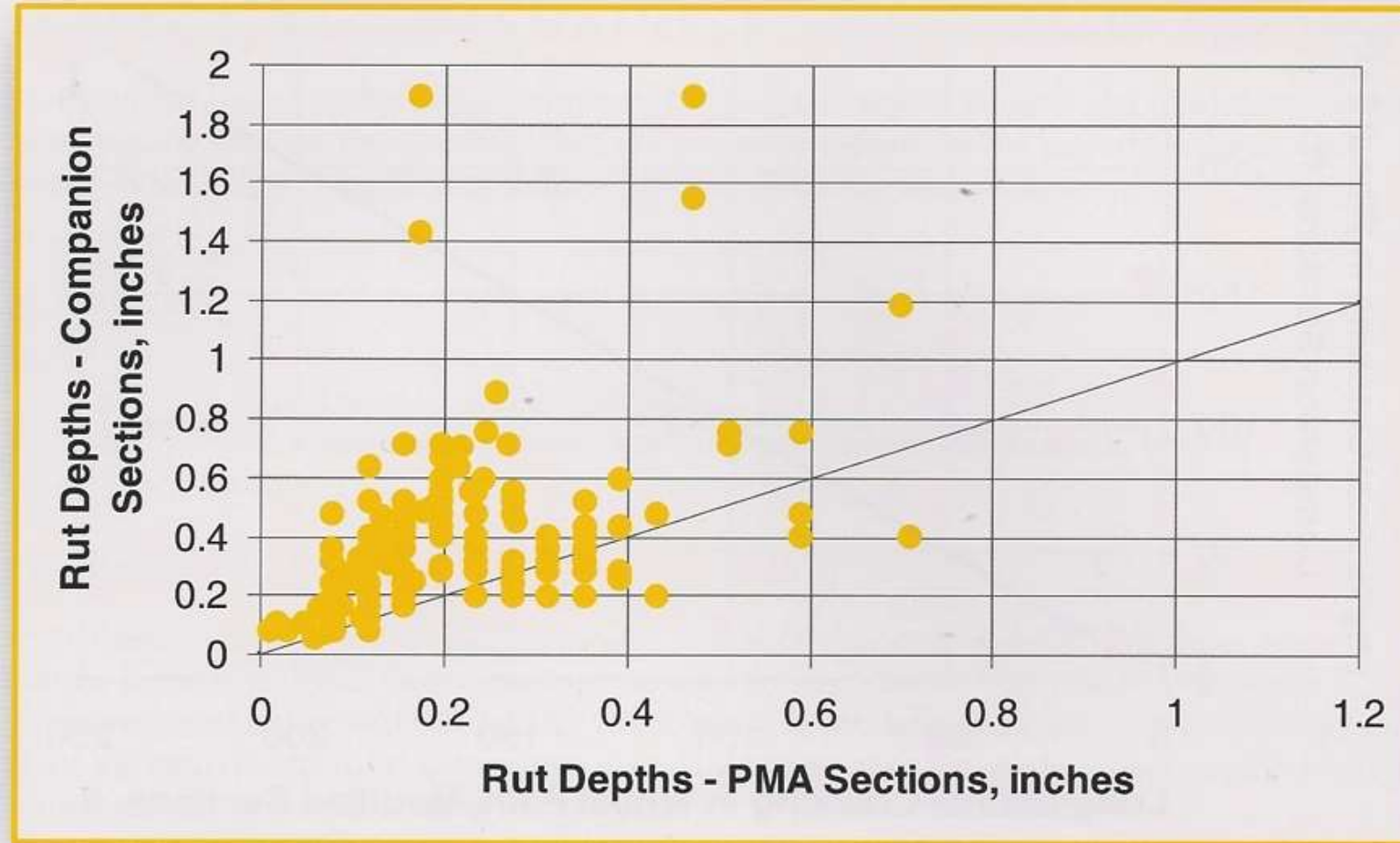


Figure 1

Graphical comparison of the rut depths measured on sections with PMA and the companion sections – those without PMA mixtures.

IS-215 Fatigue Cracking Data

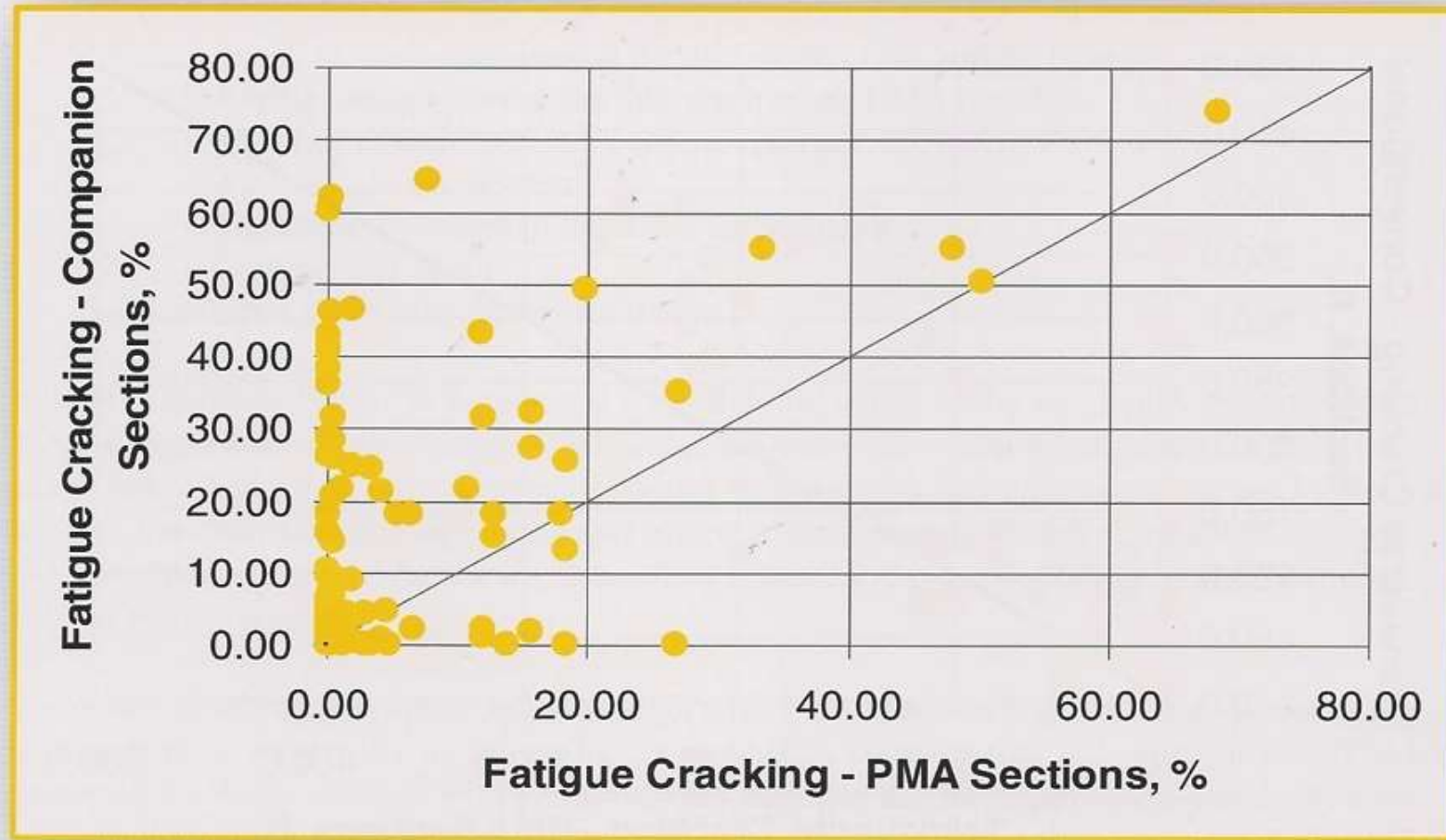


Figure 2

Graphical comparison of the fatigue cracking measured in percent wheel path area on sections with PMA and the companion sections – those without PMA mixtures.

IS-215 Thermal Cracking Data

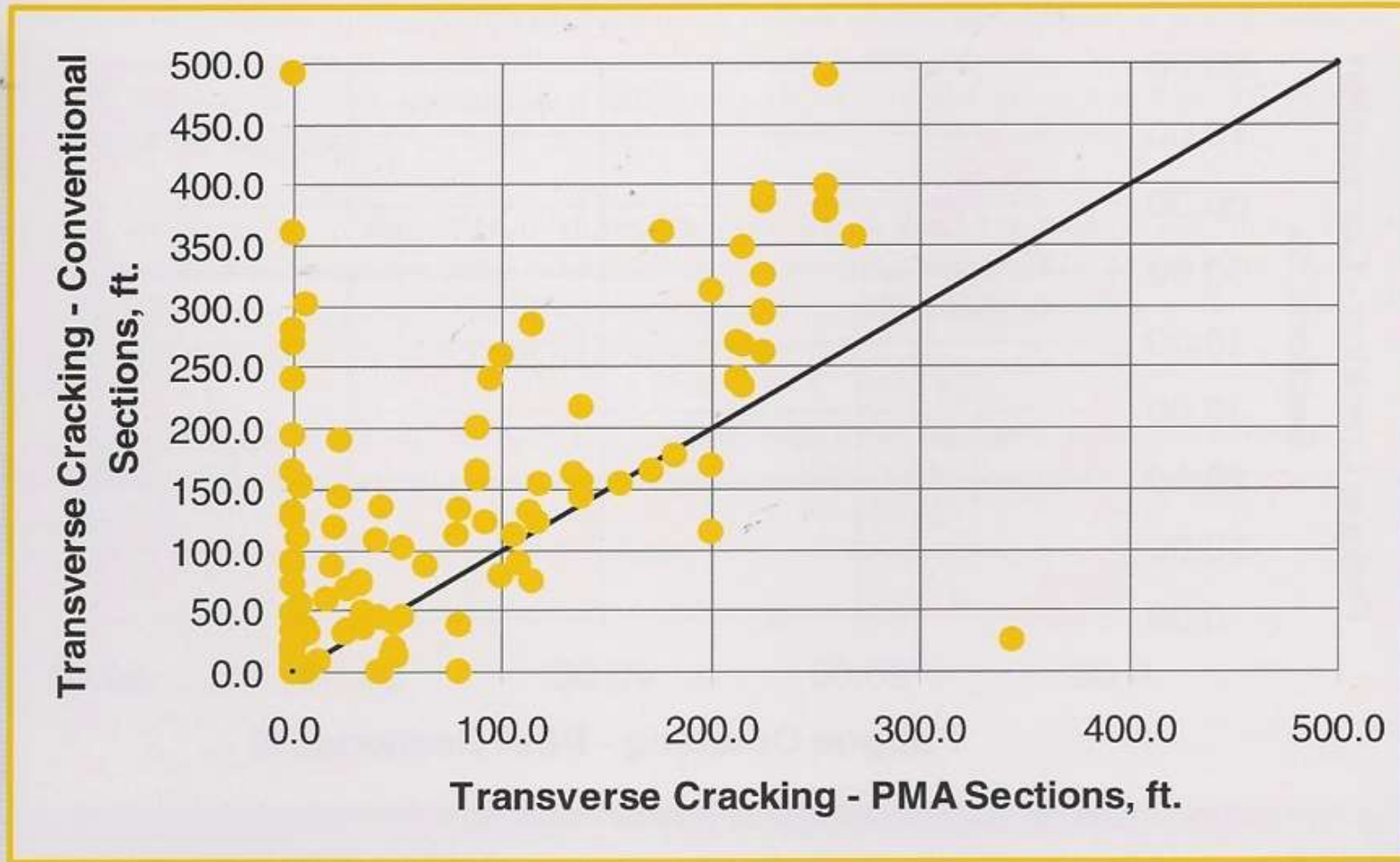


Figure 4

Graphical comparison of the length of transverse cracks measured on sections with PMA and the companion sections – those without PMA mixtures.

Summary of Expected Increase in Service Life, Years, Based on M-E Damage Based Analysis

Assumptions: Unmodified sections designed for 20 yr. life. Also, PMA in top 4 inches.

Site Factor	Condition Description		Added Life
Foundation	Non-expansive, coarse soils		5-10
	Expansive and plastic soils (PI>35)		2-5
	Frost Susceptible in cold climate		2-5
Water Table & Drainage	Deep		5-10
	Shallow; adequate		5-8
	Shallow; inadequate		0-2
Existing Pavement Condition	HMA	Good	5-10
		Poor-extensive cracking	1-3
	PCC	Good	3-6
		Poor-faulting & cracking	0-2

Continued: Summary of Expected Increase in Service Life, Yrs



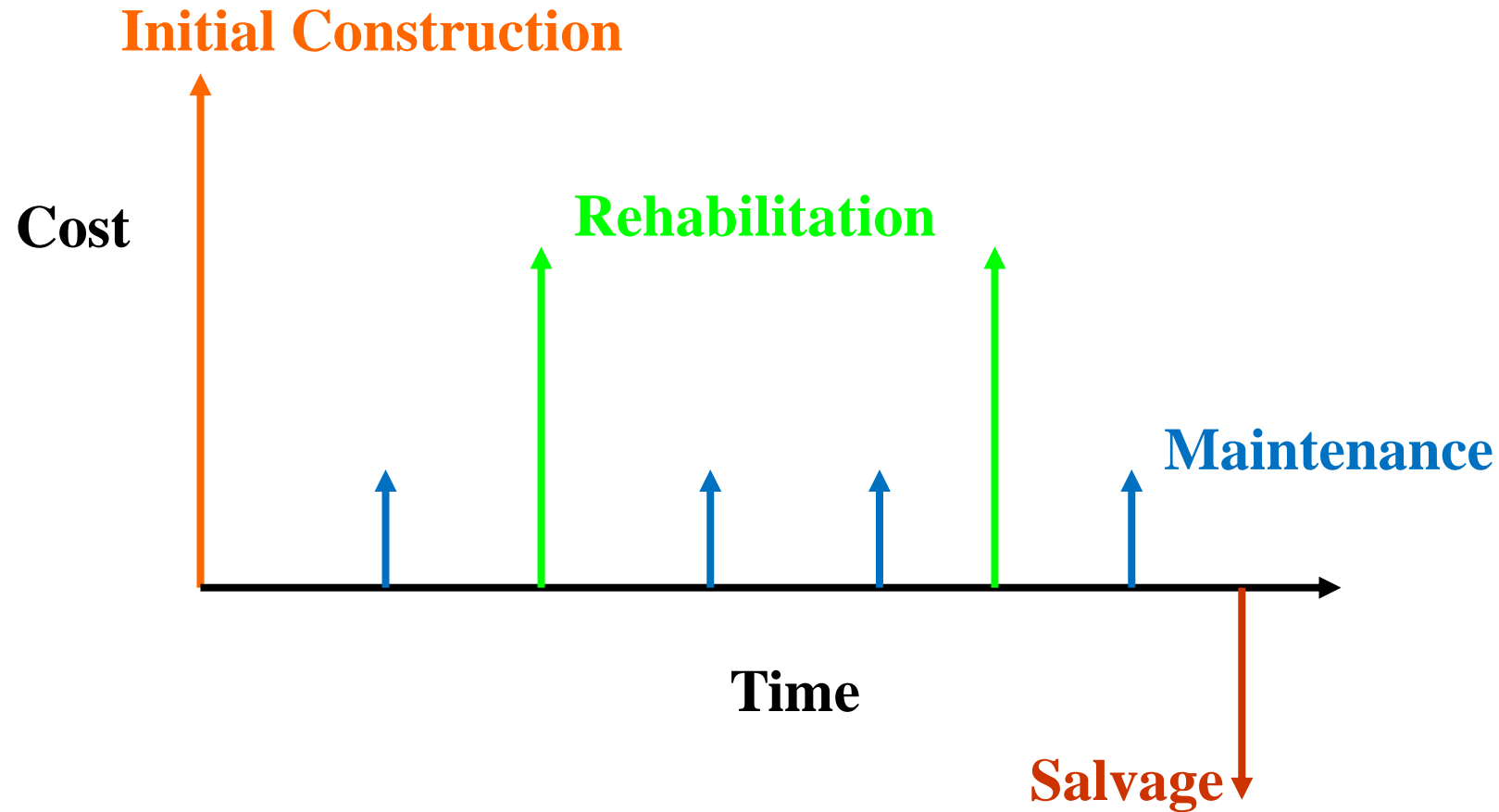
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Site Factor	Condition Description		Added Life
Climate; Temp. Fluctuations	Hot	Hot Extremes	5-10
	Mild		2-5
	Cold	Cold Extremes	3-6
Traffic, Truck Volumes	Low	Intersections	5-10
		Thoroughfares	3-6
		Heavy Loads	5-10
	Moderate		5-10
	High		5-10

Is this reliable info?

- Yes, the information, analysis and results are accurate
- These projects were part of SHRP's LTPP program
- They were well designed and well built
- It shows what we can achieve when we do things right.

We make a lot of assumptions in our business



We need to:

- Have accurate condition ratings
- Account for pavement materials
- Have uniform construction processes

Another Way?

Is there another way to analyze value?

I believe there is.

When was the last time you painted your house or fence?

What do binders cost?

2020 NDDOT Ave. Unit Bid Prices

Description		Unit	Total Quantity	Average Bid Price
PG 58S-28	Asphalt Binder	Ton	40,033	\$448.80
PG 58H-28	Asphalt Binder	Ton	23,178	\$502.93
PG 58S-34	Asphalt Binder	Ton	2,439	\$508.54
PG 58H-34	Asphalt Binder	Ton	667	\$506.00
PG 58E-34	Asphalt Binder	Ton	329	\$812.98

Highest to Lowest Difference = \$59.74 or \$364.18 per binder ton?

@ 6% Binder = \$3.58 / ton of mix for 58S-34

Simple County overlay

10 mile overlay, No milling

Original Contract Amount	\$ 1,500,814.57
1,575.5 contract binder Tons @ \$462.07/ton	\$ 727,991.29
1,575.5 Tons X \$59.74 per ton increase	\$ \$94,120.37
Increase in Project Cost	6.27%

Years needed to recover additional 6.3% expenditure		
10 year assumed life	7.5	months
20 year assumed life	15.1	months

Complicated Interstate overlay

20 + mile overlay, extensive traffic control, underdrains, etc.

Original Contract Amount	\$ 22,565,024.00
12,976.4 contract binder Tons @ \$425.78/ton	\$ 5,525,138.00
12,976.4 Tons X \$59.74 per ton increase	\$ 775,210.14
Increase in Project Cost	3.44%

Years needed to recover additional 3.44% expenditure		
10 year assumed life	4	months
20 year assumed life	8	months

Small Town Urban / Grading

1 mile 3-Lane, grading, C&G, sidewalks, lighting and new asphalt surface

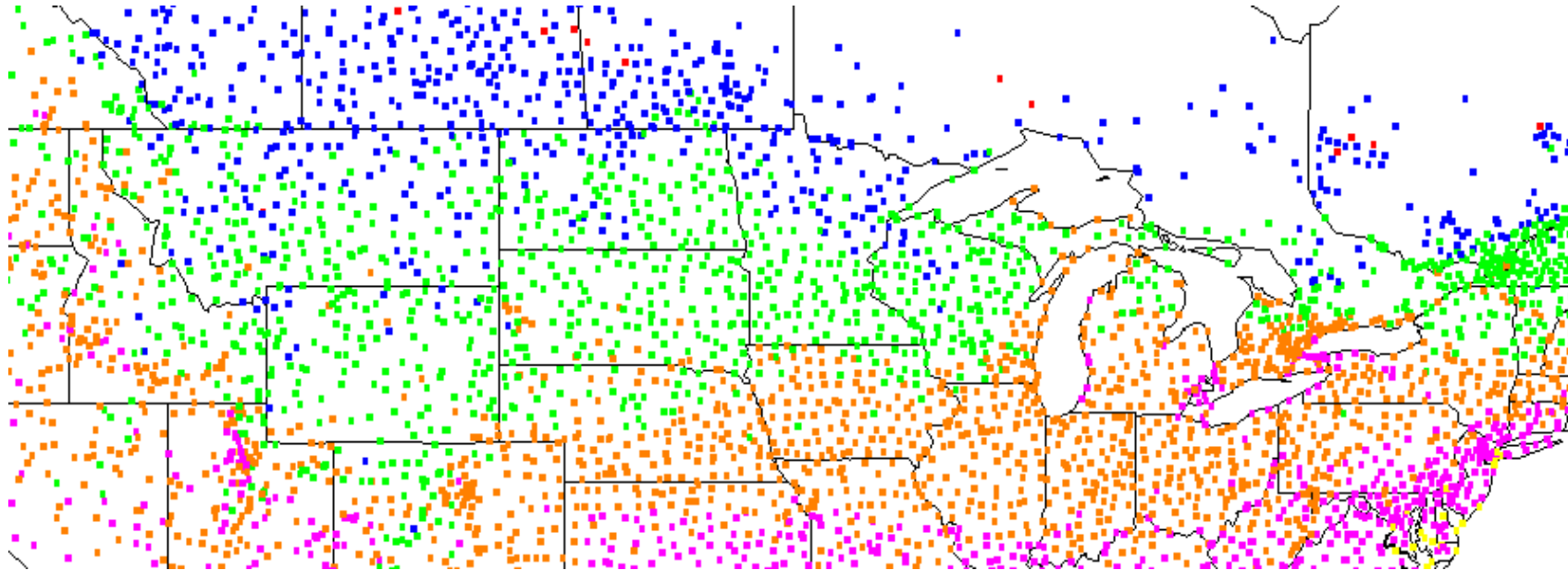
Original Contract Amount	\$3,521,707.00
9518.5 Ton recycled mix bid @ 4.7% binder	
447.7 contract binder Tons @ \$730.00/ton	\$326,821.00
571.1 (6%) virgin binder Tons X \$789.74 per ton	\$451,020.51
Project Binder cost increase	\$124,199.51
Increase in Project Cost	3.52%

Years needed to recover additional 3.52% expenditure		
10 year assumed life	4.2	months
20 year assumed life	8.4	months

***A few parting thoughts on
increasing pavement
performance***

New Construction

- Use the proper -xx binders to prevent thermal cracking!
- 25 year old SHRP recommendation.



LTPPBind v3.1

https://infopave.fhwa.dot.gov/Page/Index/LTPP_BIND

Consider higher performance binders on overlays

- Reduced rutting
- Reduced cracking
- Better crack seal performance
- Maintain existing crack resistance

Example - Do not place an overlay with -22 binder on a pavement built with a -34 binder.

Better Joint Performance

Interstate Saw and Seal

- Orig. 1962 PCC
- 1999 - 3" HMA
- 2004 – 1.5" SMA
 - ✓ PG 70-28
- 2004 – Saw/Seal
- 2009 – Sealed secondary cracks
- Photo taken in 2018



Consider reducing the # of grades

2020 NDDOT Ave. Unit Bid Prices

430	5803	PG 58S-28 ASPHALT CEMENT	46,033.00	\$20,659,811.90	\$448.80 TON
430	5806	PG 58H-28 ASPHALT CEMENT	23,178.80	\$11,657,341.53	\$502.93 TON
430	5815	PG 58S-34 ASPHALT CEMENT	2,439.03	\$1,240,338.00	\$508.54 TON
430	5818	PG 58H-34 ASPHALT CEMENT	8,987.50	\$4,547,712.50	\$506.00 TON
430	5824	PG 58E-34 ASPHALT CEMENT	667.00	\$542,258.75	\$812.98 TON
430	5828	PG 58-28 ASPHALT CEMENT	329.00	\$172,725.00	\$525.00 TON

2020 SDDOT Ave. Unit Bid Prices

320E0005	PG 58-34 Asphalt Binder	Ton	42,859.80	23,960,780.48	559.05
320E0007	PG 64-28 Asphalt Binder	Ton	871.50	579,453.00	664.89
320E0008	PG 64-34 Asphalt Binder	Ton	28,463.80	17,062,417.81	599.44

Thank you

GLOBAL MEMBERS



REGULAR MEMBERS



ASSOCIATE MEMBERS



CANADIAN MEMBERS



INTERNATIONAL MEMBERS



COMMERCIAL MEMBERS



AFFILIATE MEMBERS

